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Forced vibration analysis of laminated composite plates under the action of a moving vehicle

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ABSTRACT. This paper provides a finite element analysis of laminated composite plates under the action of a moving vehicle. The vehicle is modeled as a rigid body with four suspension systems, each consisting of a spring-dashpot. Overall, the vehicle possesses three degrees of freedom: vertical, rolling, and pitching motions. The equations of motion of the plate are deduced based on first-order shear deformation theory. Using the Euler-Lagrange equations, the system of coupled equations of motion is extracted and solved by using the Newmark time discretization scheme. The algorithm is validated through the comparison of both the free and forced vibration results provided by the present model and exact or numerical results reported in the literature. The effects are investigated of several system parameters on the dynamic response.

KEYWORDS. Forced Vibration; Laminated Composites; Moving Vehicle.

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INTRODUCTION

In recent years, the dynamic analysis of engineering structures, such as bridges, roads, and rails, under the action of moving loads has gained great attention. Such structures are often subjected to high stresses and experience severe vibrations. Bridges as main substructures can be modeled as plates traversed by three major types of loading: moving loads, moving masses, and moving oscillators. Thus, researchers have studied their behavior under the action of these loadings.

Some researchers have focused their studies on analyzing the effects of moving loads on isotropic beams and plates [1-4], while others have focused on the case of moving masses [5-8]. Besides, some works are related to moving oscillators [9-11]. Ghafoori et al. [12] provided a semi-analytical method to obtain the dynamic response of a plate subjected to a moving oscillator. Also, Wu et al. [13] aimed at introducing a technique to replace each 3-DOF system consisting of spring-mass by a set of equivalent masses. Lin and Trethewey [14] investigated the response of elastic beams subjected to an arbitrary spring-mass damper system and obtained the governing equations of motion based on the finite element method (FEM).

Recently, laminated composites, due to their lightweight and high strength, as well as the material adaptability, have gained in popularity for the construction of civil structures such as bridges. This has brought a new field of interest in studying the response of either laminated composite plates or beams traversed by moving oscillators or loads. Malekzadeh et al. [15] studied the dynamic response of cross-ply thick laminated plate under the action of moving load based on three-dimensional elasticity. They applied layerwise theory to discretize the equations of motion. Mohebpour et al. [16] investigated the response of laminated composite plate subjected to moving oscillator using the FEM based on first-order shear deformation theory (FSDT). In 2004, Lee et al. [17] analyzed a multi-span continuous composite plate under multi-moving loads based on third-order shear deformation theory (TSDT). Ghafoori and Asghari [18] presented an analysis of angle-ply laminated composite plates traversed by moving masses and forces. They applied the FEM to obtain equations of motion and solved them by using the Newmark method. Mohebpour et al. [19] developed an algorithm based on the FEM to study the response of laminated composite beams subjected to moving oscillators. They used FSDT to obtain the equations of the beam.

Kim [20] studied the dynamic stability behavior of damped laminated beams subjected to uniformly distributed forces based on a finite element formulation consistent with Vlasov's beam theory. Also, the effect of fiber orientation, boundary conditions, and external and internal damping was studied. It should be mentioned that the dynamic response of an intact plate can be used for damage detection in a defected plate [21-23].

In this paper, the problem of a laminated composite plate subjected to a moving vehicle is investigated. Thus, the effects of various parameters, such as vehicle mass, plate damping ratio, etc., are also investigated. The governing equations of the plate are obtained based on FSDT and the vehicle is considered as a rigid body having 3 degrees of freedom: vertical, rolling, and pitching motions. This modelling approach is the major novelty of the present paper. Lastly, the Newmark time integration procedure is used to find the response of the system in time.

MATHEMATICAL MODELING

Consider a laminated composite plate under the action of a moving vehicle with constant velocity V along the x -axis as shown in Fig. 1. The plate has length a , width b and thickness h with the coordinate frame placed at the mid-plane. The displacement field based on FSDT is as follows:

$$\begin{aligned} u(x, y, z, t) &= u_0(x, y, t) + z\psi_x(x, y, t) \\ v(x, y, z, t) &= v_0(x, y, t) + z\psi_y(x, y, t) \\ w(x, y, z, t) &= w_0(x, y, t) \end{aligned} \quad (1)$$

where u , v and w are the displacements of a point of the laminate (x, y, z) in the three coordinate directions. Also, u_0 , v_0 and w_0 refer to displacements of a point on the mid-plane ($z=0$) and ψ_x , ψ_y refer to rotations about the x - and y -axis, respectively. Using Eqn.1, the non-zero strain components are derived as follow:

$$\begin{aligned} \epsilon_x = u_{,x} &= u_{0,x} + z\psi_{x,x} = \epsilon_x^0 + z\kappa_x \\ \epsilon_y = v_{,y} &= v_{0,y} + z\psi_{y,y} = \epsilon_y^0 + z\kappa_y \\ \gamma_{xy} = u_{,y} + v_{,x} &= u_{0,y} + v_{0,x} + z\psi_{x,y} + z\psi_{y,x} = \gamma_{xy}^0 + z\kappa_{xy} \end{aligned} \quad (2)$$

$$+2B_{66}(\psi_{x,y} + \psi_{y,x})(u_{0,y} + v_{0,x}) + 2D_{12}\psi_{y,y}\psi_{x,x} + 2D_{16}(\psi_{x,y} + \psi_{y,x})\psi_{x,x} \\ + 2D_{26}(\psi_{x,y} + \psi_{y,x})\psi_{y,y} + 2A_{45}(\psi_x + w_{0,x})(\psi_y + w_{0,y})] dy dx$$

Furthermore, the kinetic energy of the laminated composite plate is:

$$T_p = \frac{1}{2} \int_0^a \int_0^b [I_0(u_t^2 + v_t^2 + w_t^2) + 2I_1(u_t\psi_{x,t} + v_t\psi_{y,t}) + I_2(\psi_{x,t}^2 + \psi_{y,t}^2)] dy dx \quad (6)$$

where (I_0, I_1, I_2) are mass moments of inertia, defined as follows:

$$(I_0, I_1, I_2) = \int_{-b/2}^{b/2} \rho(\tilde{z})(1, \tilde{z}, \tilde{z}^2) d\tilde{z} \quad (7)$$

As mentioned earlier, a moving vehicle is modeled with 3-DOF as shown in Fig. 1. The potential energy of the vehicle is:

$$U_V = \frac{1}{2} k_1 (q_1 - w_1 - b_V q_2 + a_V q_3)^2 + \frac{1}{2} k_2 (q_1 - w_2 - b_V q_2 - a_V q_3)^2 \\ + \frac{1}{2} k_3 (q_1 - w_3 + b_V q_2 + a_V q_3)^2 + \frac{1}{2} k_4 (q_1 - w_4 + b_V q_2 - a_V q_3)^2 + \frac{mg}{4} (w_1 + w_2 + w_3 + w_4) \quad (8)$$

where

$$w_i = w_0(x_i, y_i) \quad (i=1,2,3,4) \quad (9)$$

Also, the damped energy of dashpots can be written as:

$$W_{dv} = \frac{1}{2} c_1 (\dot{q}_1 - \dot{w}_1 - b_V \dot{q}_2 + a_V \dot{q}_3)^2 + \frac{1}{2} c_2 (\dot{q}_1 - \dot{w}_2 - b_V \dot{q}_2 - a_V \dot{q}_3)^2 \\ + \frac{1}{2} c_3 (\dot{q}_1 - \dot{w}_3 + b_V \dot{q}_2 + a_V \dot{q}_3)^2 + \frac{1}{2} c_4 (\dot{q}_1 - \dot{w}_4 + b_V \dot{q}_2 - a_V \dot{q}_3)^2 \quad (10)$$

In the above equations, k_i and c_i ($i=1,2,3,4$) refer to the suspension system stiffness and damping parameters, respectively. Moreover, the kinetic energy of the vehicle is:

$$T_V = \frac{1}{2} m \dot{q}_1^2 + \frac{1}{2} I_x \dot{q}_2^2 + \frac{1}{2} I_y \dot{q}_3^2 \quad (11)$$

where m is the vehicle total mass and I_x and I_y are its mass moments of inertia about the x - and y -axes respectively.

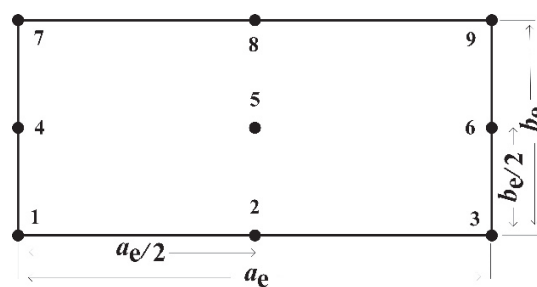


Figure 2: Rectangular higher-order element



FINITE ELEMENT SOLUTION

In order to obtain numerical results, we propose a higher-order plate element as shown in Fig. 2. The element has 9 nodes and each node has 5 degrees of freedom including the axial displacement u_0 , lateral displacements v_0 , w_0 , and independent rotations ψ_x , ψ_y .

To obtain the generalized displacement corresponding to each degree of freedom inside an element, the Lagrange interpolation is used. This can be stated as:

$$\begin{aligned} u_0(x, y) &= [N_u] \{d\} \\ v_0(x, y) &= [N_v] \{d\} \\ w_0(x, y) &= [N_w] \{d\} \\ \psi_x(x, y) &= [N_{\psi_x}] \{d\} \\ \psi_y(x, y) &= [N_{\psi_y}] \{d\} \end{aligned} \quad (12)$$

where $\{d\}$ is the element nodal displacement vector and $[N_u]$, $[N_v]$, $[N_w]$, $[N_{\psi_x}]$, and $[N_{\psi_y}]$ are the shape function matrices, defined as:

$$\begin{aligned} [N_u] &= [N_1 \ 0 \ 0 \ 0 \ 0 \ N_2 \ 0 \ 0 \ 0 \ 0 \ \dots \ N_9 \ 0 \ 0 \ 0 \ 0] \\ [N_v] &= [0 \ N_1 \ 0 \ 0 \ 0 \ 0 \ N_2 \ 0 \ 0 \ 0 \ \dots \ 0 \ N_9 \ 0 \ 0 \ 0] \\ [N_w] &= [0 \ 0 \ N_1 \ 0 \ 0 \ 0 \ 0 \ N_2 \ 0 \ 0 \ \dots \ 0 \ 0 \ N_9 \ 0 \ 0] \\ [N_{\psi_x}] &= [0 \ 0 \ 0 \ N_1 \ 0 \ 0 \ 0 \ 0 \ N_2 \ 0 \ \dots \ 0 \ 0 \ 0 \ N_9 \ 0] \\ [N_{\psi_y}] &= [0 \ 0 \ 0 \ 0 \ N_1 \ 0 \ 0 \ 0 \ 0 \ N_2 \ 0 \ \dots \ 0 \ 0 \ 0 \ 0 \ N_9] \end{aligned} \quad (13)$$

where the N_i functions are

$$\begin{aligned} N_1 &= (\zeta - 1)(2\zeta - 1)(\eta - 1)(2\eta - 1) \\ N_2 &= -4\zeta(\zeta - 1)(\eta - 1)(2\eta - 1) \\ N_3 &= \zeta(2\zeta - 1)(\eta - 1)(2\eta - 1) \\ N_4 &= -4(\zeta - 1)(2\zeta - 1)\eta(\eta - 1) \\ N_5 &= 16\zeta(\zeta - 1)\eta(\eta - 1) \end{aligned} \quad (14)$$



$$N_6 = -4\zeta(2\zeta - 1)\eta(\eta - 1)$$

$$N_7 = (\zeta - 1)(2\zeta - 1)\eta(2\eta - 1)$$

$$N_8 = -4\zeta(\zeta - 1)\eta(2\eta - 1)$$

$$N_9 = \zeta(2\zeta - 1)\eta(2\eta - 1)$$

Here, $\zeta = \frac{x}{a_e}$ and $\eta = \frac{y}{b_e}$ are non-dimensional element coordinates and a_e and b_e are the element length and width, respectively.

Substituting Eqn.12 into Eqn.5 and Eqn.6 respectively provides:

$$U_p = \frac{1}{2} \{d\}^T [K_e] \{d\} \quad (15)$$

$$T_p = \frac{1}{2} \{\dot{d}\}^T [M_e] \{\dot{d}\} \quad (16)$$

where $[K_e]$ is the element stiffness matrix and $[M_e]$ is the element mass matrix. Their expressions are provided in Appendix A. After assembling the element matrices and applying the Euler-Lagrange equations, the coupled governing equations of motion are obtained as follows:

$$[M] \begin{Bmatrix} \ddot{\Delta} \\ q_1 \\ q_2 \\ q_3 \end{Bmatrix} + [C] \begin{Bmatrix} \dot{\Delta} \\ \dot{q}_1 \\ \dot{q}_2 \\ \dot{q}_3 \end{Bmatrix} + [K] \begin{Bmatrix} \Delta \\ q_1 \\ q_2 \\ q_3 \end{Bmatrix} = \{f\} \quad (17)$$

where $\{\Delta\}$ is the plate total displacement vector and $[C_p]$ is the plate damping matrix, assumed as Rayleigh's proportional damping [25]:

$$[C_p] = a_0 [M_p] + a_1 [K_p] \quad (\text{a } 18)$$

$$\frac{1}{2} \begin{bmatrix} 1/\omega_i & \omega_i \\ 1/\omega_j & \omega_j \end{bmatrix} \begin{Bmatrix} a_0 \\ a_1 \end{Bmatrix} = \begin{Bmatrix} \xi_i \\ \xi_j \end{Bmatrix} \quad (\text{b } 18)$$

Eqn.17 is discretized by applying the Newmark time integration method in which $\gamma = 1/2$ and $\beta = 1/4$.

NUMERICAL RESULTS

In this section, firstly, the free vibration results are compared with those available in the literature. Also, a parametric analysis is carried out to study the effects of system dynamic characteristics on the dynamic response of plate. The examples provided are based on the following material properties unless mentioned otherwise:

$$E_1 = 40 \text{ GPa}, \quad E_2 = 9.65 \text{ GPa}, \quad G_{12} = G_{13} = 0.6 E_2$$

$$G_{23} = 0.5 E_2, \quad \nu_{12} = 0.25, \quad \rho = 1389.23 \left(\text{kg} / \text{m}^3 \right)$$

As a first example, the variation of the non-dimensional natural frequency of a laminated composite plate simply supported along all edges with symmetric cross-ply layup ($0^\circ / 90^\circ$) is considered by changes of a / b . Tab. 1 shows the results.

Refs.	a / b			
	10	20	50	100
Kant et al. [26]	15.1048	17.6470	18.6720	18.835
Matsunaga [27]	15.0721	17.6369	18.6702	18.835
Reddy [28]	15.1073	17.6457	18.6718	18.835
Akavci [29]	15.3684	17.7584	18.6934	18.841
Rodriguez et al. [30]	15.1674	17.7471	18.7895	18.956
Abedi et al. [31]	15.1056	17.6448	18.6719	18.836
Present	15.1425	17.6592	18.6689	18.789

Table 1: Non-dimensional natural frequency ($\Omega = (\omega a^2 / h) \sqrt{\rho / E_{22}}$) of a laminated composite plate simply supported at all edges with symmetric cross-ply layup ($0^\circ / 90^\circ$).

From the above results, it is obvious that an increment in the plate length with respect to its thickness decreases the overall stiffness of the plate. As a consequence, it increases the non-dimensional natural frequency.

The next example expresses the effect of plate length to thickness on the first three non-dimensional natural frequencies. The plate has a ($0^\circ / 30^\circ / 60^\circ / 0^\circ$) layup and two alternative boundary conditions: CCCC and SSSS.

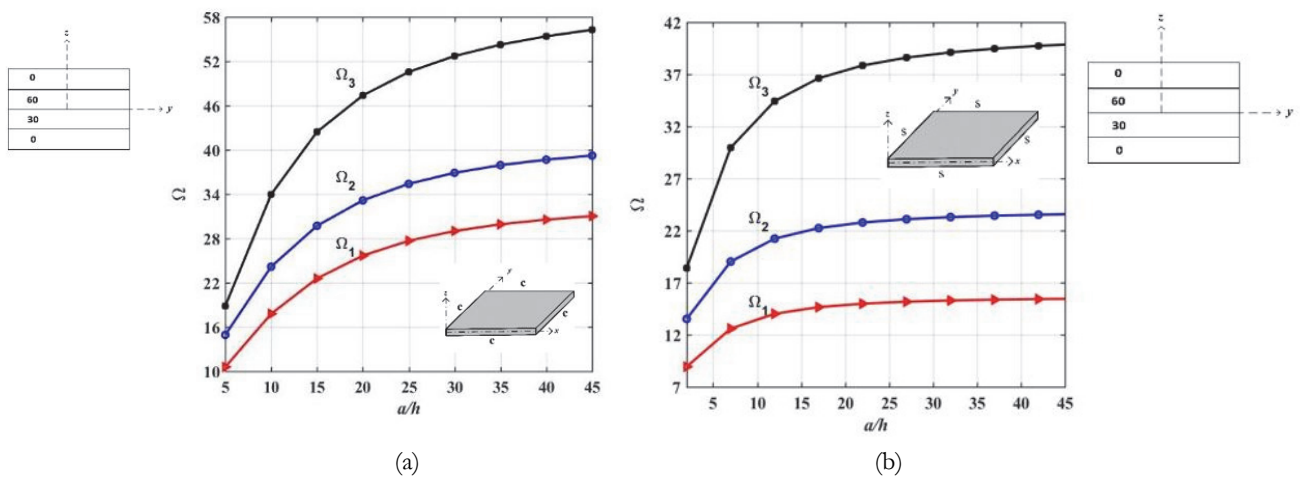


Figure 3: Effect of plate length to thickness on the first three non-dimensional natural frequencies (a) CCCC (b) SSSS. ($E_1 = 25 E_2$, $G_{12} = G_{13} = 0.5 E_2$, $G_{23} = 0.2 E_2$)

In order to obtain non-dimensional forced vibration results, firstly, non-dimensional parameters are discussed as below:

(a) velocity parameter α , defined as the ratio of the fundamental period of the plate to the time required for the vehicle passing the span

$$\alpha = \frac{2\pi V}{\omega_p a} \quad (19)$$

where V is the velocity of vehicle. It is supposed that the velocity is constant and the vehicle is moving along the x -axis.

(b) mass parameter κ , defined as the ratio of vehicle mass to the plate mass

$$\kappa = \frac{m}{M_p} = \frac{m}{\rho a b h} \quad (20)$$

(c) frequency parameter γ_i , defined as the ratio of the natural frequency of quarter-vehicle to the fundamental frequency of the plate

$$\gamma_i = \frac{\omega_i}{\omega_p} = \frac{\sqrt{\frac{k_i}{m/4}}}{\omega_p} \quad (i = 1, 2, 3, 4) \quad (21)$$

(d) mass moments of inertia ϵ and χ are defined as below:

$$\chi = \frac{I_y}{M_p a^2} \quad (\text{a } 22)$$

$$\epsilon = \frac{I_x}{M_p b^2} \quad (\text{b } 22)$$

(e) logarithmic decrement of quarter-vehicle spring-dashpot Δ_i , which is defined as

$$\Delta_i = \frac{c_i}{2\left(\frac{m}{4}\right)^f} = \frac{c_i}{2\left(\frac{m}{4}\right)\left(\frac{1}{2\pi}\sqrt{\frac{k_i}{m/4}}\right)} \quad (i = 1, 2, 3, 4) \quad (23)$$

In the following examples, the values of k_i and c_i for all suspension systems are equal.

To validate the forced vibration results, the model of the vehicle is reduced to a moving oscillator as discussed in [16]. Fig. 4 shows the comparison of results. It can be seen that the present reduced-order model tracks the reported results with low deviation. Therefore, the maximum errors are 2.29% for $\gamma = 1.5$ and 2.17% for $\gamma = 2$.

The next examples provide a better look at the mid-point deflection of the laminated composite plate. As a first example, the effect of boundary conditions on the dynamic magnification factor (DMF), defined as the ratio of maximum dynamic deflection with respect to maximum static deflection is studied. A laminated plate is considered with $(30^\circ / 60^\circ)_{AS}$ layup.

Here, the subscript “AS” refers to an anti-symmetric stacking sequence. Four different boundary conditions are considered: CCCC, SSSS, CFCF, and SFSF. It should be noted that, for a better comparison among results, the maximum static deflection, in this example and in the further ones, is referred to the condition having lower DMF. It can be concluded that the CCCC boundary condition has higher stiffness. Thus, it gains lower amounts of DMF, as can be seen from Fig. 5.

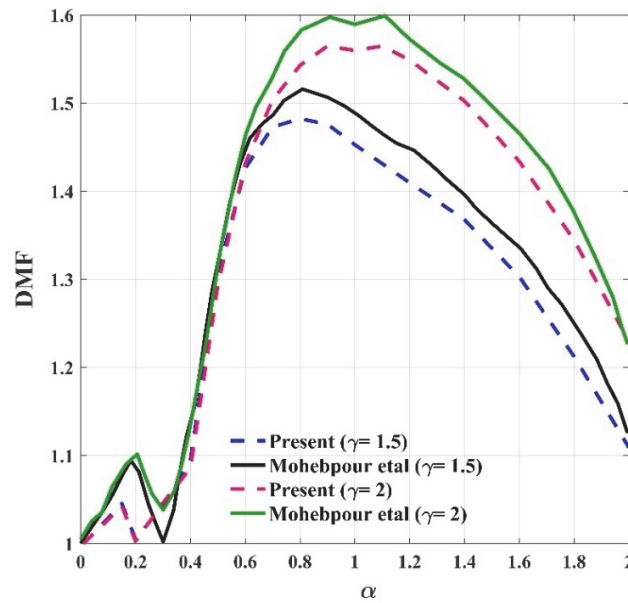


Figure 4: Comparison of present reduced-order model with reference [16]

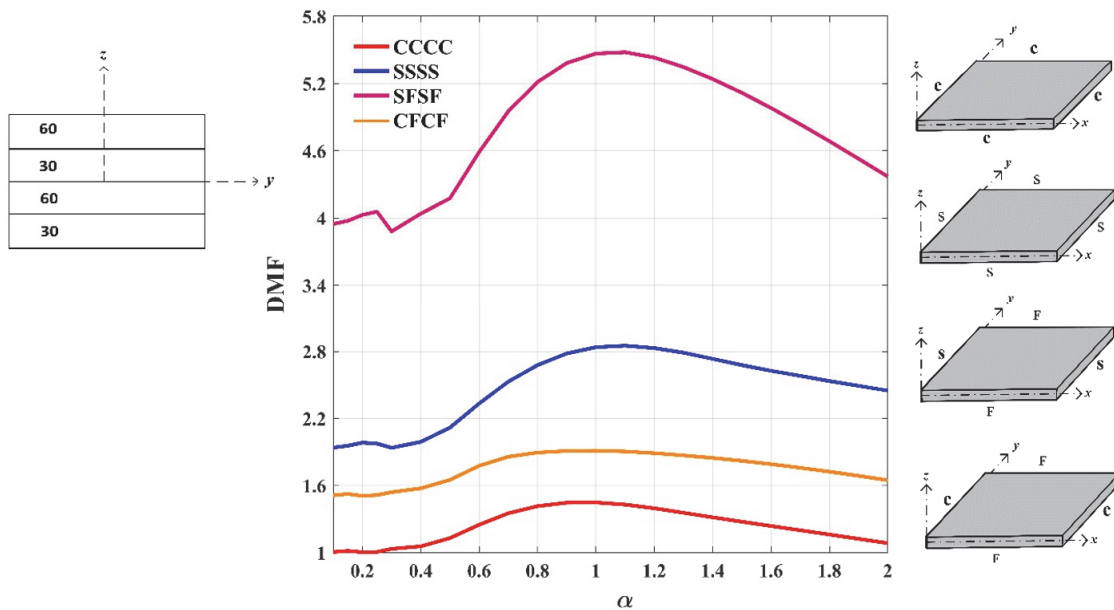


Figure 5: Influence of various boundary conditions on mid-point DMF ($\kappa=0.2$, $\gamma=0.1$, $\chi=\epsilon=0.1$, $\Delta=0.1$, $\xi_p=5\%$).

Fig. 6 presents the effect of the plate slenderness ratio parameter under CCCC boundary condition with $(30^\circ/60^\circ)_{AS}$ layup. Increasing the length causes DMF to shift towards higher values. This happens because an increase in the length of the plate leads to a decrement in its structural stiffness and an increment in the dynamic deflection. To study the effect of plate damping ratio, consider a plate simply supported along all edges with $(30^\circ/60^\circ)_{AS}$ layup. According to Fig. 7 increasing damping ratio leads to a reduction in mid-point dynamic deflection which indeed reduces DMF. However, the critical velocity is kept constant. The last example expresses the influence of vehicle mass on the dynamic deflection of the mid-point. The laminated composite plate has $(0^\circ/30^\circ/60^\circ/0^\circ)$ layup with CCCC boundary condition. Fig. 8 shows that, as long as the vehicle

mass increases, the mid-point dynamic deflection increases during the existence of loading on plate. As a matter of fact, the DMF increases as shown in Fig. 9.

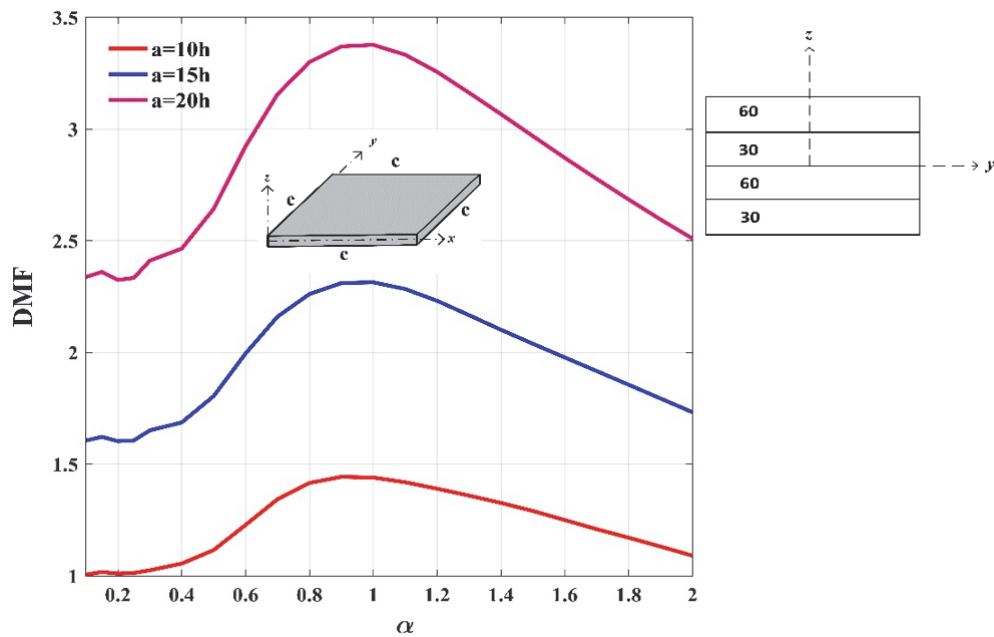


Figure 6: Influence of slenderness ratio on mid-point DMF ($\kappa = 0.2$, $\gamma = 0.1$, $\chi = \epsilon = 0.1$, $\Delta = 0.1$, $\xi_p = 5\%$).

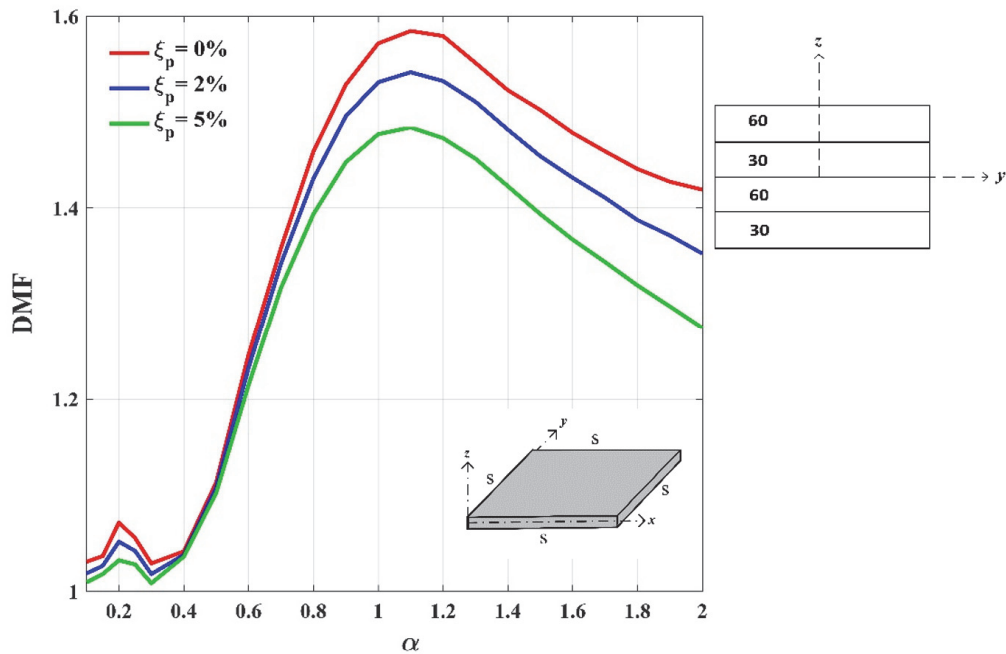


Figure 7: Effect of plate damping ratio on mid-point DMF ($\kappa = 0.2$, $\gamma = 0.1$, $\chi = \epsilon = 0.1$, $\Delta = 0.1$).

CONCLUSION

In this paper, the forced vibrations are investigated of a laminated composite plate under the action of a moving vehicle. Several examples are included to study the influence of system dynamic parameters, such as plate damping ratio, vehicle mass, and plate slenderness ratio. The equations of motion of plate are obtained based on first-order shear deformation

theory and solved using Newmark's discretization scheme. Free and forced vibration results show good agreement with those available in the literature. This modeling algorithm can be extended to a full vehicle model to obtain results that are applicable in practical designs. It is shown that the SFSF boundary condition provides higher values of DMF. Besides, decreasing the damping ratio and increasing the slenderness ratio, alongside with the vehicle mass, can have considerable effect on the DMF.

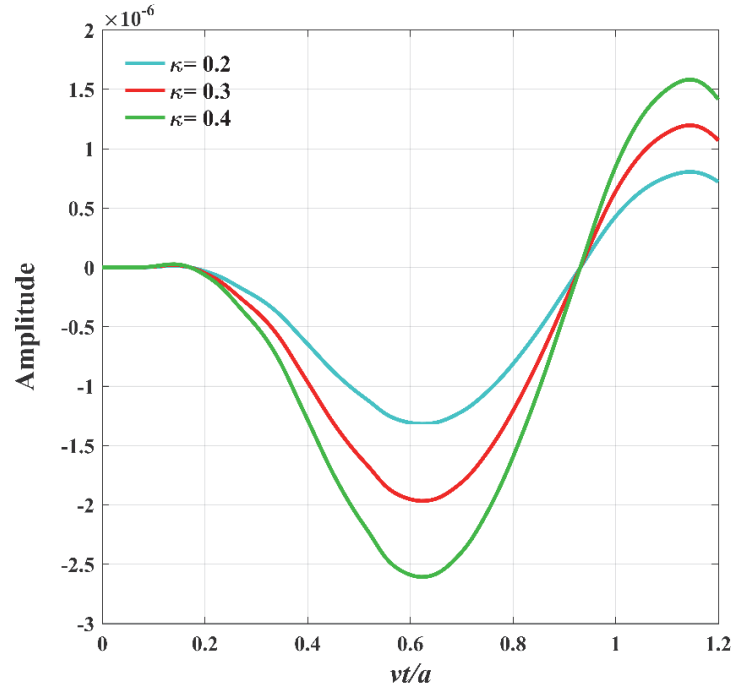


Figure 8: Effect of vehicle mass on mid-point dynamic deflection during the existence of load on plate ($\kappa = 0.2$, $\gamma = 0.1$, $\chi = \varepsilon = 0.1$, $\Delta = 0.1$, $\xi_p = 5\%$, $\alpha = 1$)

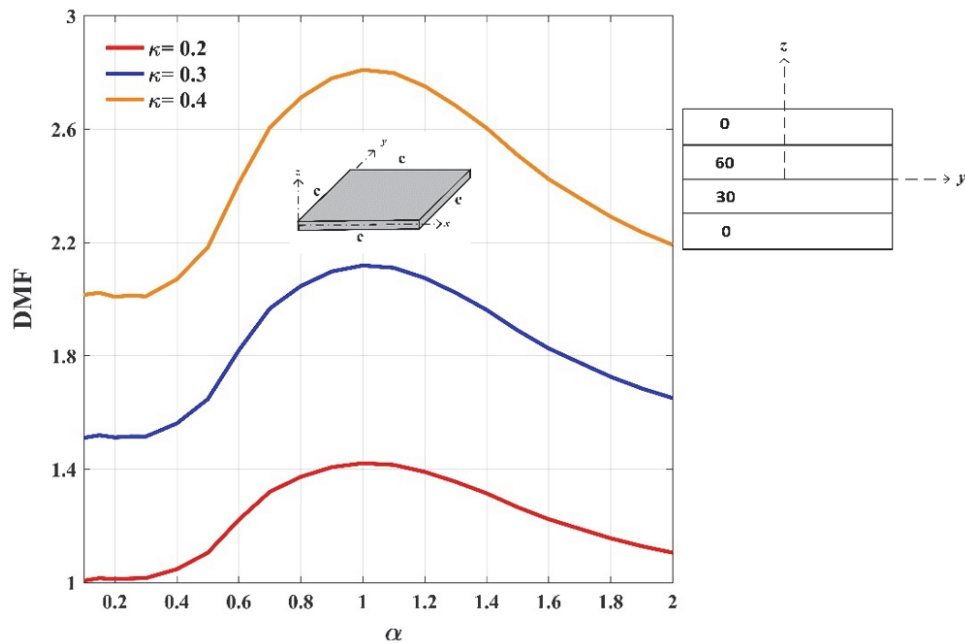


Figure 9: Dynamic magnification factor for various vehicle masses ($\kappa = 0.2$, $\gamma = 0.1$, $\chi = \varepsilon = 0.1$, $\Delta = 0.1$, $\xi_p = 5\%$)

APPENDIX A

$$\begin{aligned}
[K_e] = & \int_0^a \int_0^b [A_{11} N_{u_0,x}^T \cdot N_{u_0,x} + A_{22} N_{v_0,y}^T \cdot N_{v_0,y} + A_{66} (N_{u_0,y}^T \cdot N_{u_0,y} + N_{v_0,x}^T \cdot N_{v_0,x} \\
& + N_{u_0,y}^T \cdot N_{v_0,x} + N_{v_0,x}^T \cdot N_{u_0,y}) + D_{11} N_{\psi_{x,x}}^T \cdot N_{\psi_{x,x}} + D_{22} N_{\psi_{y,y}}^T \cdot N_{\psi_{y,y}} \\
& + D_{66} (N_{\psi_{x,y}}^T \cdot N_{\psi_{x,y}} + N_{\psi_{y,x}}^T \cdot N_{\psi_{y,x}} + N_{\psi_{x,y}}^T \cdot N_{\psi_{y,x}} + N_{\psi_{y,x}}^T \cdot N_{\psi_{x,y}} \\
& + N_{\psi_{y,x}}^T \cdot N_{\psi_{x,y}}) + A_{44} (N_{\psi_y}^T \cdot N_{\psi_y} + N_{w_0,y}^T \cdot N_{w_0,y} + N_{\psi_y}^T \cdot N_{w_0,y} \\
& + N_{w_0,y}^T \cdot N_{\psi_y}) + A_{55} (N_{\psi_x}^T \cdot N_{\psi_x} + N_{w_0,x}^T \cdot N_{w_0,x} + N_{\psi_x}^T \cdot N_{w_0,x} \\
& + N_{w_0,x}^T \cdot N_{\psi_x}) + A_{12} (N_{u_0,x}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{u_0,x}) + A_{16} (N_{u_0,y}^T \cdot N_{u_0,x} \\
& + N_{u_0,x}^T \cdot N_{u_0,y} + N_{v_0,x}^T \cdot N_{u_0,x} + N_{u_0,x}^T \cdot N_{v_0,x}) + B_{11} (N_{\psi_{x,x}}^T \cdot N_{u_0,x} + N_{u_0,x}^T \cdot N_{\psi_{x,x}}) \\
& + B_{12} (N_{\psi_{y,y}}^T \cdot N_{u_0,x} + N_{u_0,x}^T \cdot N_{\psi_{y,y}}) + B_{16} (N_{\psi_{x,y}}^T \cdot N_{u_0,x} + N_{u_0,x}^T \cdot N_{\psi_{x,y}} \\
& + N_{\psi_{y,x}}^T \cdot N_{u_0,x} + N_{u_0,x}^T \cdot N_{\psi_{y,x}}) + A_{26} (N_{u_0,y}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{u_0,y} \\
& + N_{v_0,x}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{v_0,x}) + B_{12} (N_{\psi_{x,x}}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{\psi_{x,x}}) \\
& + B_{22} (N_{\psi_{y,y}}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{\psi_{y,y}}) + B_{26} (N_{\psi_{x,y}}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{\psi_{x,y}} \\
& + N_{\psi_{y,x}}^T \cdot N_{v_0,y} + N_{v_0,y}^T \cdot N_{\psi_{y,x}}) + B_{16} (N_{\psi_{x,x}}^T \cdot N_{u_0,y} + N_{u_0,y}^T \cdot N_{\psi_{x,x}} \\
& + N_{\psi_{x,x}}^T \cdot N_{v_0,x} + N_{v_0,x}^T \cdot N_{\psi_{x,x}}) + B_{26} (N_{\psi_{y,y}}^T \cdot N_{u_0,y} + N_{u_0,y}^T \cdot N_{\psi_{y,y}} \\
& + N_{\psi_{y,y}}^T \cdot N_{v_0,x} + B_{66} (N_{\psi_{x,y}}^T \cdot N_{u_0,y} + N_{u_0,y}^T \cdot N_{\psi_{x,y}} + N_{\psi_{x,y}}^T \cdot N_{v_0,x} \\
& + N_{v_0,x}^T \cdot N_{\psi_{x,y}} + N_{\psi_{y,x}}^T \cdot N_{u_0,y} + N_{u_0,y}^T \cdot N_{\psi_{y,x}} + N_{\psi_{y,x}}^T \cdot N_{v_0,x} \\
& + N_{v_0,x}^T \cdot N_{\psi_{y,x}}) + D_{12} (N_{\psi_{y,y}}^T \cdot N_{\psi_{x,x}} + N_{\psi_{x,x}}^T \cdot N_{\psi_{y,y}}) + D_{16} (N_{\psi_{x,y}}^T \cdot N_{\psi_{x,x}} \\
& + N_{\psi_{x,x}}^T \cdot N_{\psi_{x,y}} + N_{\psi_{y,x}}^T \cdot N_{\psi_{x,x}} + N_{\psi_{x,x}}^T \cdot N_{\psi_{y,x}}) + D_{26} (N_{\psi_{x,y}}^T \cdot N_{\psi_{y,y}} \\
& + N_{\psi_{y,y}}^T \cdot N_{\psi_{x,y}} + N_{\psi_{y,y}}^T \cdot N_{\psi_{y,x}} + N_{\psi_{y,x}}^T \cdot N_{\psi_{y,y}}) + A_{45} (N_{\psi_x}^T \cdot N_{\psi_y} \\
& + N_{\psi_y}^T \cdot N_{\psi_x} + N_{\psi_x}^T \cdot N_{w_0,y} + N_{w_0,y}^T \cdot N_{\psi_x} + N_{w_0,x}^T \cdot N_{\psi_y} + N_{\psi_y}^T \cdot N_{w_0,x} \\
& + N_{w_0,x}^T \cdot N_{w_0,y} + N_{w_0,y}^T \cdot N_{w_0,x})] dy dx
\end{aligned}$$

$$\begin{aligned}
[M_e] = & \int_0^a \int_0^b [I_0 ([N_u]^T \cdot [N_u] + [N_v]^T \cdot [N_v] + [N_w]^T \cdot [N_w]) + I_1 ([N_u]^T \cdot [N_{\psi_x}] + [N_{\psi_x}]^T \cdot [N_u] \\
& + [N_v]^T \cdot [N_{\psi_y}] + [N_{\psi_y}]^T \cdot [N_v]) + I_2 ([N_{\psi_x}]^T \cdot [N_{\psi_x}] + [N_{\psi_y}]^T \cdot [N_{\psi_y}])] dy dx
\end{aligned}$$

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